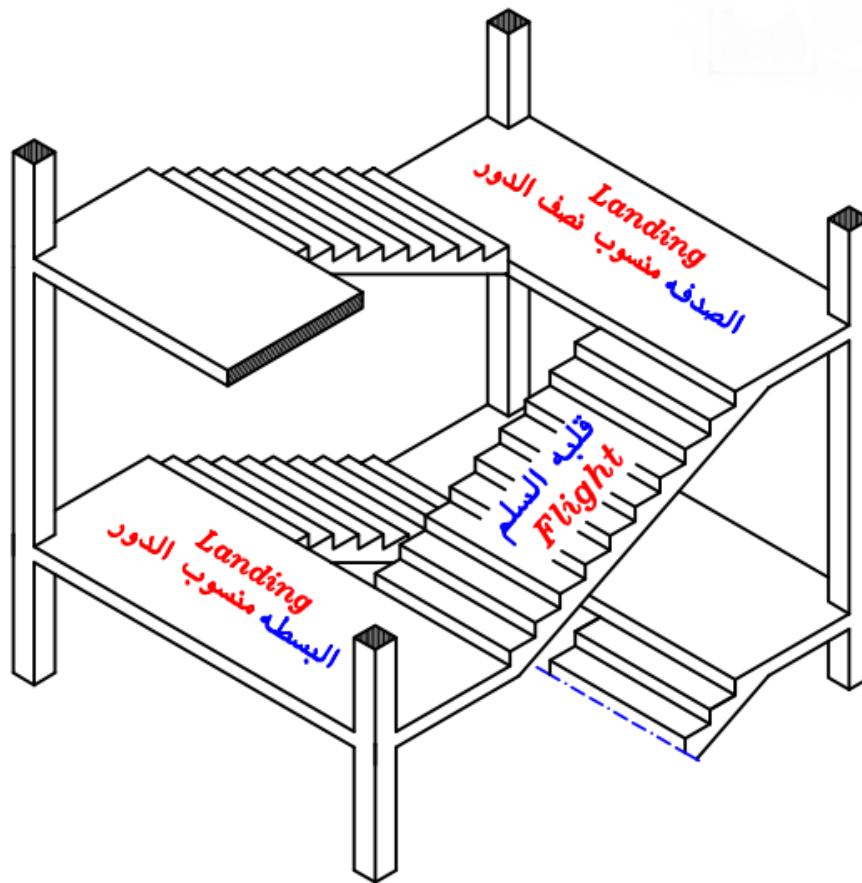


Design Of Stairs

نosalكم الدعاء

Steps of Design STAIRS .



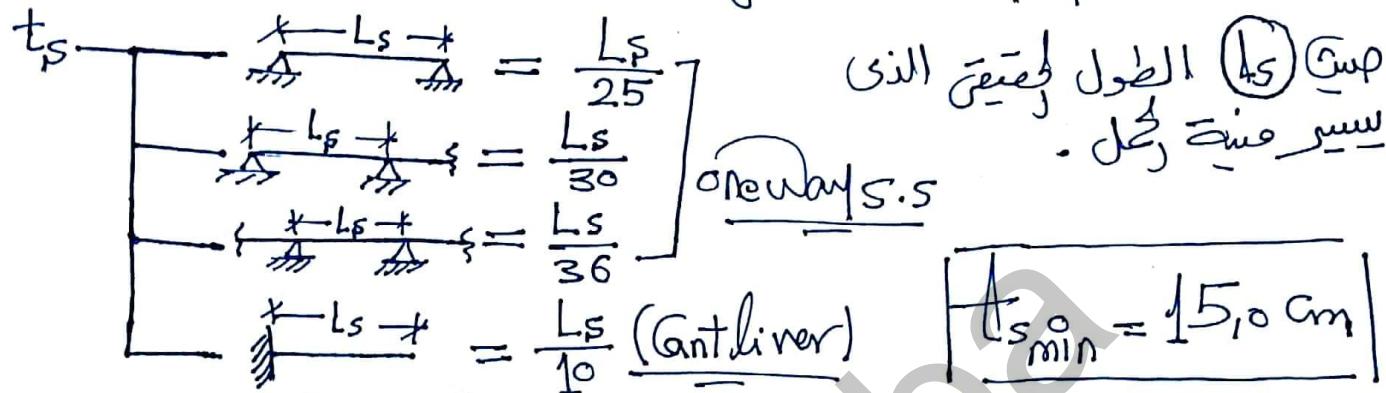
DESIGN OF STAIRS

Steps of Design Stairs -

amr Tolba

① Concrete Dims - (After statical system of stairs)

one way solid slab $\text{مُبَلَّغٌ بِلِسْنِهِ أَكْلَهُ}$



② Load Calculation

$$w_s = 1.4 D \cdot L + 1.6 L \cdot L$$

$$\begin{aligned} D \cdot L &\rightarrow HZ = t_s \gamma_c + F.c \\ &\rightarrow \gamma_{nc} = t_{an} \gamma_c + F.c \end{aligned}$$

$$\begin{aligned} L \cdot L &\rightarrow HZ = l \cdot l \\ &\rightarrow \gamma_{nc} = l \cdot l \cdot G_s \end{aligned}$$

$$\begin{aligned} w_s &= 1.4 [t_s \gamma_c + F.c] + 1.6 l \cdot l = \dots \text{ per } m^2 \\ w_s^{\circ} &= 1.4 [t_{an} \gamma_c + F.c] + 1.6 l \cdot l \cdot G_s = \dots \text{ per } m^2 \end{aligned}$$

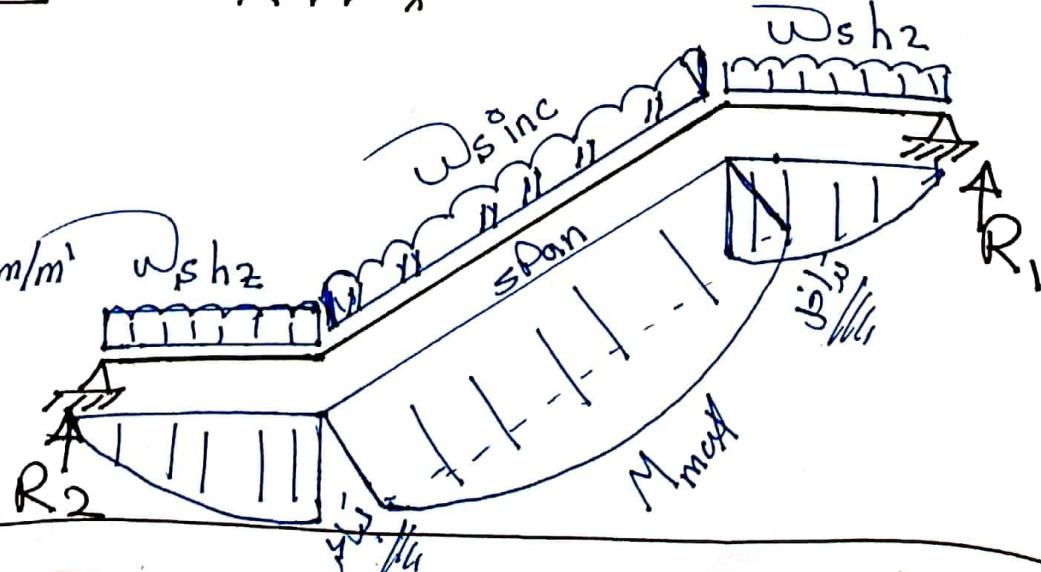
$$t_{san} = t_s + 7 \text{ cm}$$

strip 1m in load direction.

③ take steps

$$\begin{aligned} R_1 &= 1 \text{ ton} \\ R_2 &= 1 \text{ ton} \end{aligned}$$

$$M_{ult} = M_{max} = 1 \text{ t.m/m}^2$$



④ Design :- $M_{max} = \sqrt{f_c t \cdot m/m^3}$ & $t_s = 10\text{ cm}$ & $d = 15\text{ cm}$
 $F_{cu} = 1$ & $F_y = 1$

$$d = C_1 \sqrt{\frac{M_{max}}{F_{cu} \cdot b}} \rightarrow d = 100\text{ cm}$$

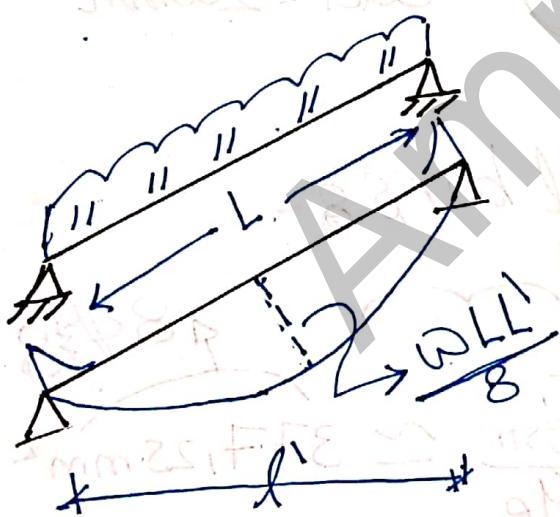
$$d = t_s - \text{cover} \rightarrow (2-3)\text{ cm}$$

$$C_1 = 1 \text{ From table } J = 1.1$$

$$A_s = \frac{M_{max}}{F_y \cdot J \cdot d} = \text{cm}^2/\text{m}^3 \rightarrow A_{min}$$

$$A_{smin} = \frac{11}{F_y} B \cdot d = 11 > 6 \text{ #}/\text{m}^2$$

⑤ Draw Reinforcement :-



$$\text{* Rise} (\bar{\omega}_L) = R = (14 \rightarrow 18\text{ cm}) \xrightarrow{\text{diff}} 15\text{ cm}$$

$$\text{* Going} (\bar{\omega}_V) = G = (26 \rightarrow 30\text{ cm}) \xrightarrow{\text{diff}} 30\text{ cm}$$

Stairs

For the shown stairs.

- F.c. = 150 kg/m^2

- L.L. = 300 kg/m^2

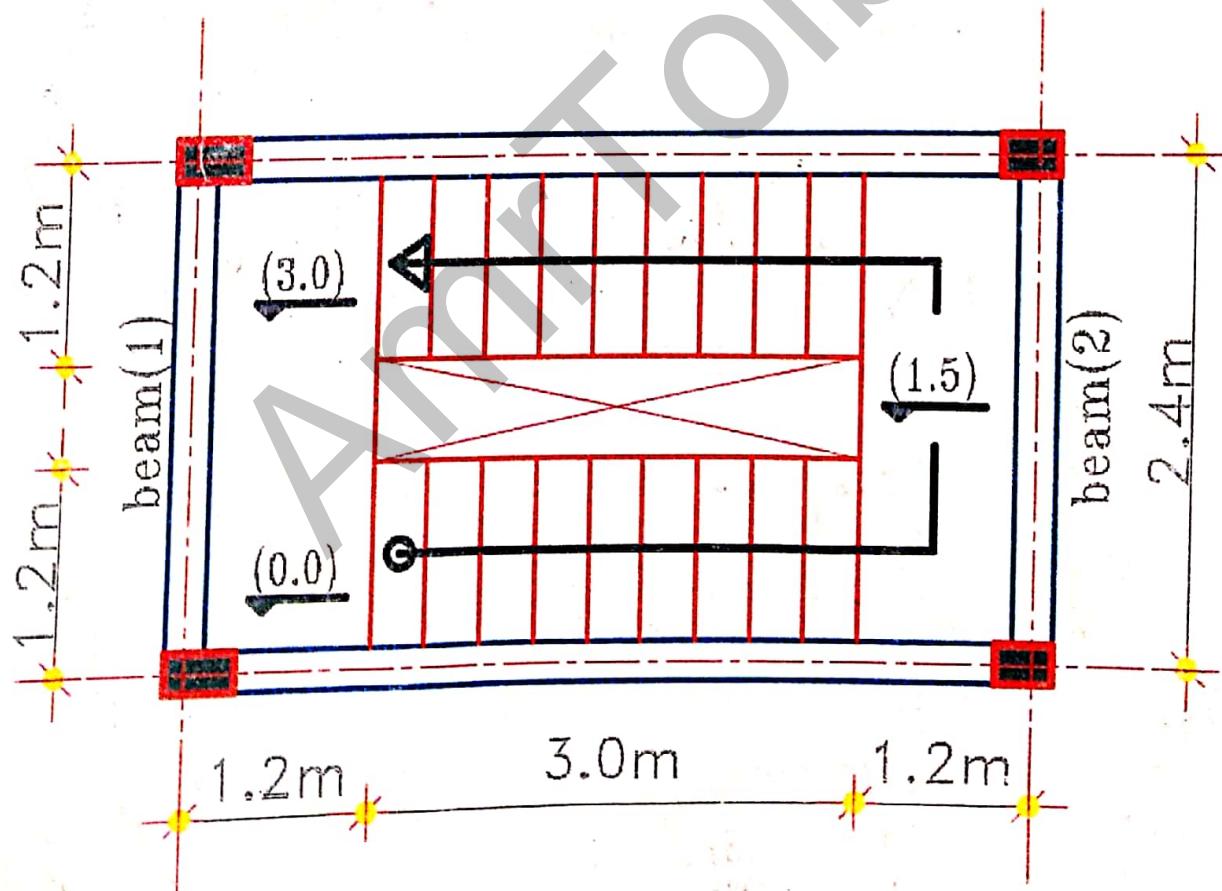
- $F_{cu} = 250 \text{ kg/cm}^2$

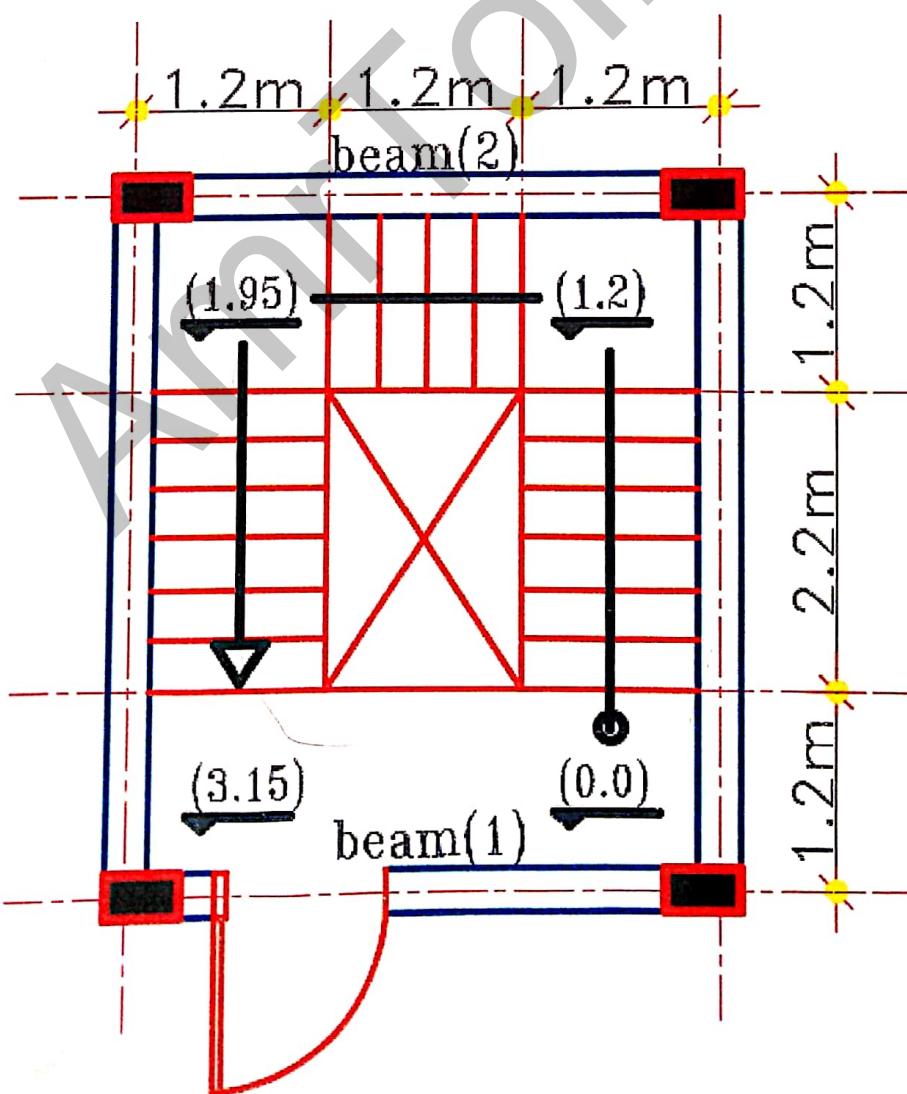
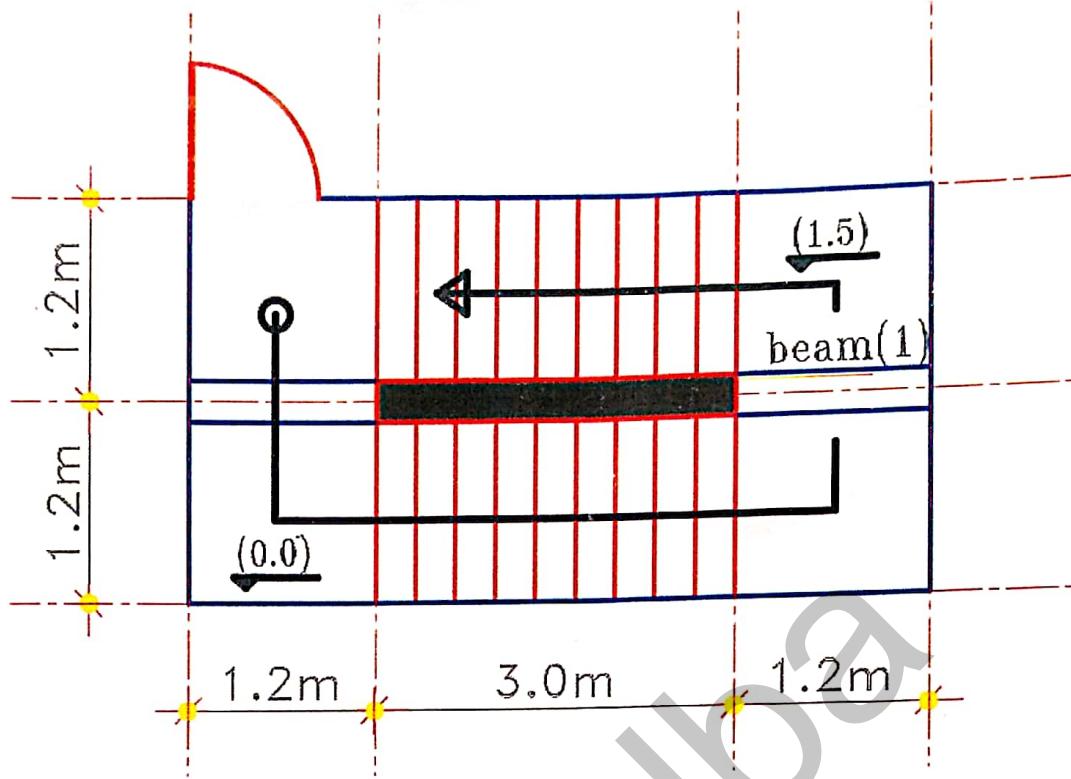
- St.36/52

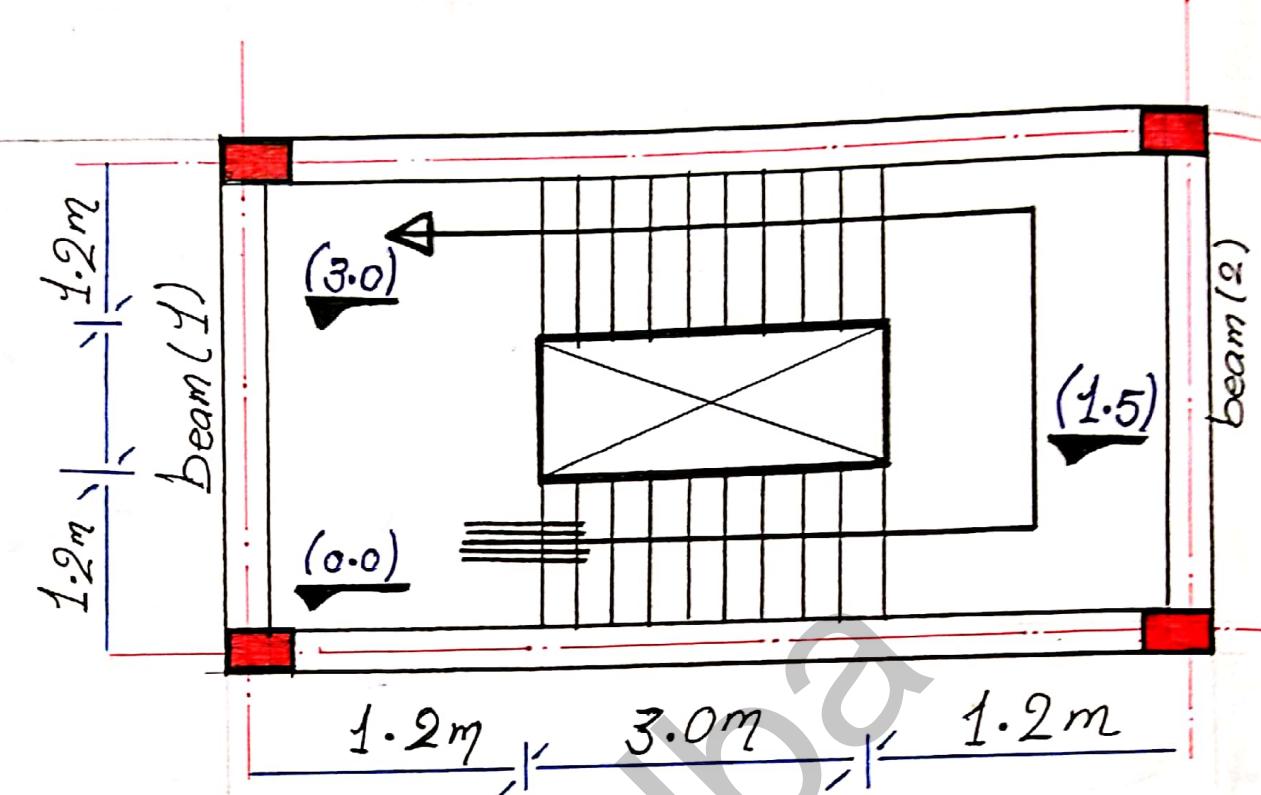
****It is required to:-**

- Design the stairs slabs and draw details of reinforcement in plan scale (1:50) and longitudinal sections.

- Design of Beams (B1-B2) and draw details of reinforcement (longitudinal and cross).







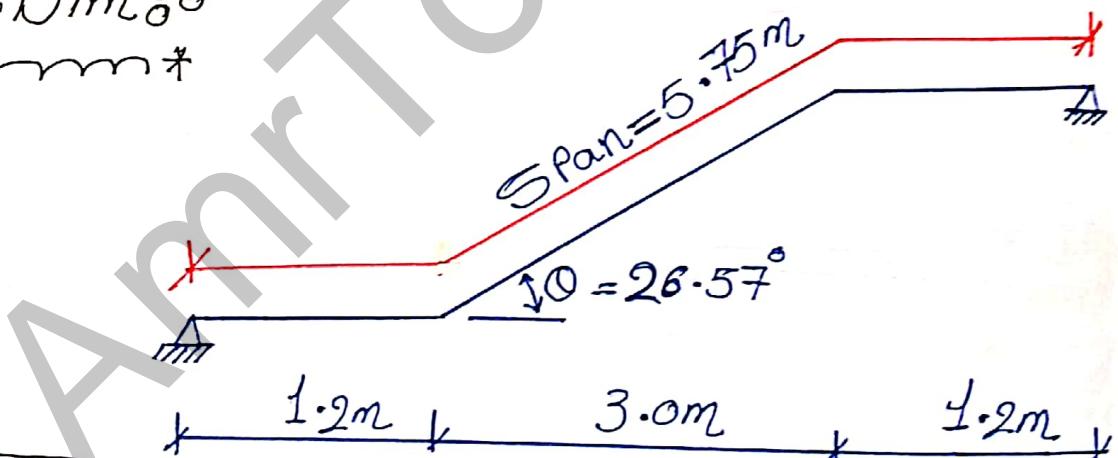
1] Concrete Dim θ
+ mm * mm *

$$t_s = \frac{\text{Span}}{25}$$

$$= \frac{5750}{25}$$

$$= 230$$

$$= 240 \text{ mm}$$



$$t_{\text{corr}} = t_s + \gamma_0 = 310 \text{ mm}$$

2] load Calculations

* * * *

$$\# \left(\frac{w_{su}}{h_z} \right)_{pz} = 1.4(t_s \gamma_c + F_c) + 1.6(l.p)$$

$$= 1.4[0.24 \times 25 + 1.5] + 1.6 \times 3.0$$

$$\frac{w_{su}}{h_z} = 15.3 \text{ kN/m}^2$$

$$\# \frac{w_{su}}{I_{nc}} = 1.4[t_s \gamma_c + F_c] + 1.6(l.p) G_s C$$

$$= 1.4[0.31 \times 25 + 1.5] + 1.6 \times 3.0 \times G_s (26.57)$$

$$\frac{w_{su}}{I_n} = 17.75 \text{ kN/m}^2$$

3) take SFD

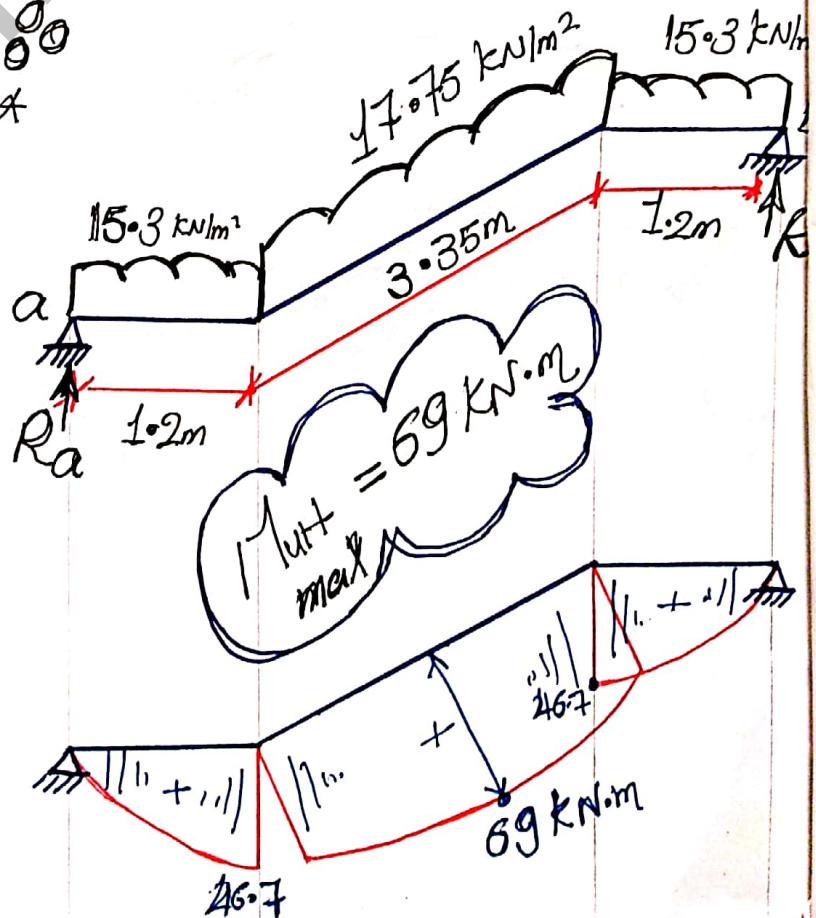
$$\# \sum F_a = 0, 0$$

$$18.36 \times 0.6 + 59.46 \times 2.7$$

$$+ 18.36 \times 4.8 = R_b \times 5.4$$

$$R_b = 48.1 \text{ kN}$$

$$R_a = 48.1 \text{ kN}$$



41

Design Of Sections

$M_{ultmax} = 69 \text{ kNm}$

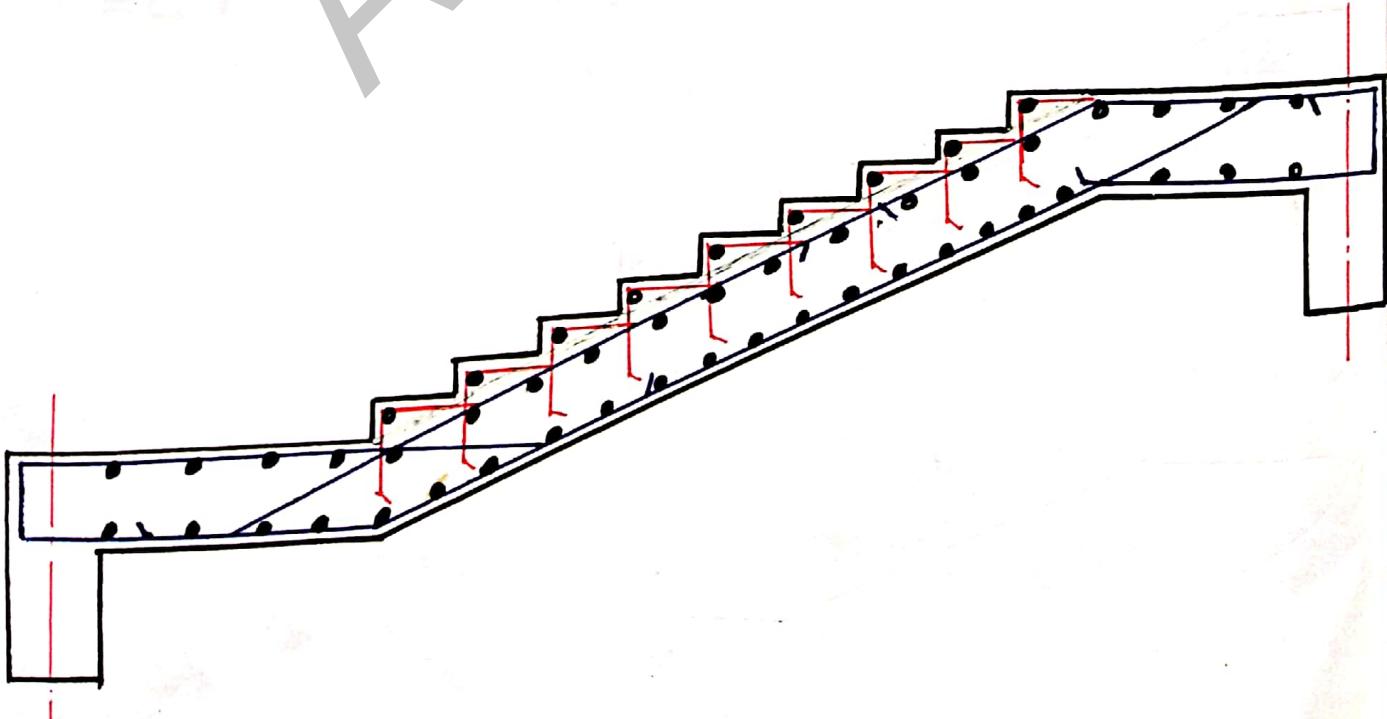
$d = 220 = q C_1 \sqrt{\frac{69 \times 10^6}{25 \times 1000}} \Rightarrow C_1 = 4.19 \quad J = 0.826 \\ J = 0.813$

$$A_s = \frac{M_{ult} \times 10^6}{F_y \cdot J \cdot d} = \frac{69 \times 10^6}{360 \times 0.813 \times 220} = 1077.6 \text{ mm}^2 \\ = 6 \# 18/\text{m}^1$$

$$A_{smin} = \frac{1.1}{F_y} b \cdot d = \frac{1.1}{360} \times 1000 \times 220 = 672 \text{ mm}^2 A_{eq} \\ \# \text{OK}$$

use 6#18/m¹

for all slabs



Design of Beam :-

~~~~~ \* ~~~~~ \*

**[B]** no torsion

V.r Loads > (250 + 600)

$$m = \alpha \cdot w + m_{all} + m_S$$

$$\alpha \cdot w = 1.4 \times 25 + 0.35 \times 0.6 = 7.35 \text{ kN/m}$$

$$m_{all} = 2 \text{ kN/m}$$

$$m_S = R = 48.1 \text{ kN/m}^3$$

#  $m = 58 \text{ kN/m}^3$

$$d = c \sqrt{\frac{M_u}{F_{cu} \cdot b}}$$

$$330 = c \sqrt{\frac{42 \times 10^6}{25 \times 600}} \Rightarrow c = 6 \quad J = 0.826$$

$$A_s = 236 \text{ mm}^2$$

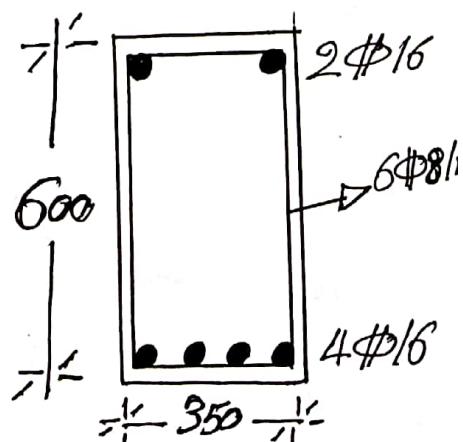
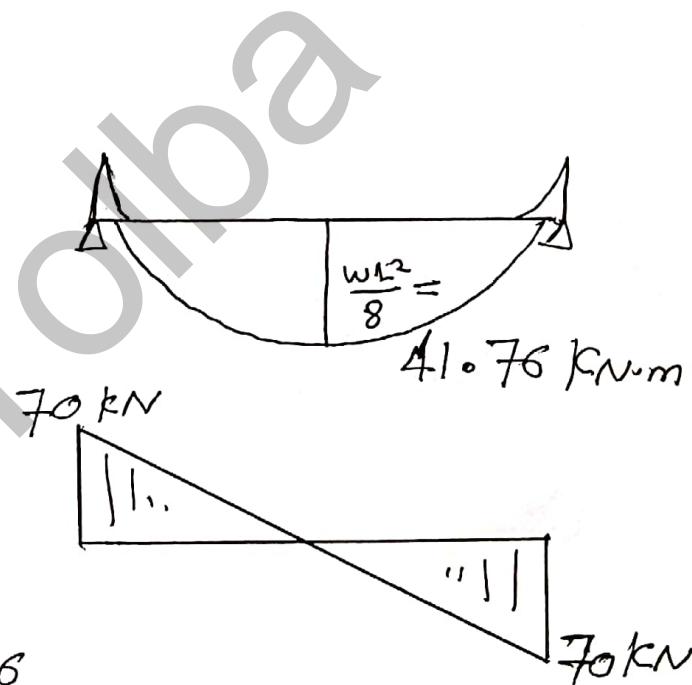
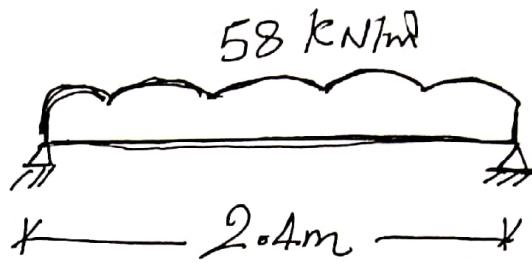
#  $A_{smin} = 642 \text{ mm}^2 = 4\#16$

Shear

$$q = \frac{Q}{B \cdot D} = \frac{70 \times 10^3}{350 \times 600} = 0.333 \text{ N/mm}^2$$

$$V_{cu} = 0.24 \sqrt{\frac{F_{cu}}{\delta_c}} = 0.979 \text{ N/mm}^2$$

use min stirrups  $8\#8/100$



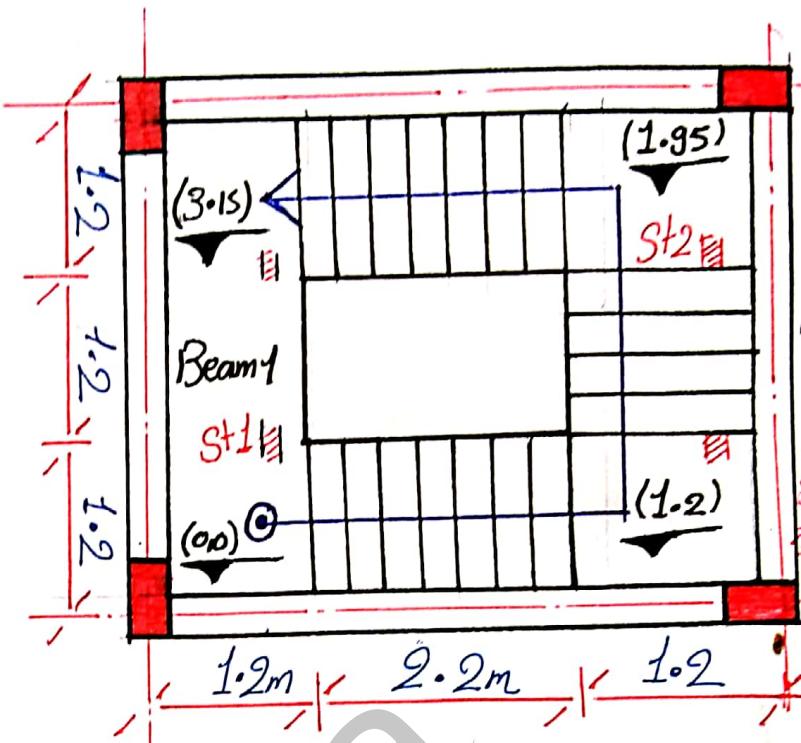
 Concrete Dimensions

$$t_s = \frac{5P_{uN}}{25.0} = \frac{4900}{25}$$

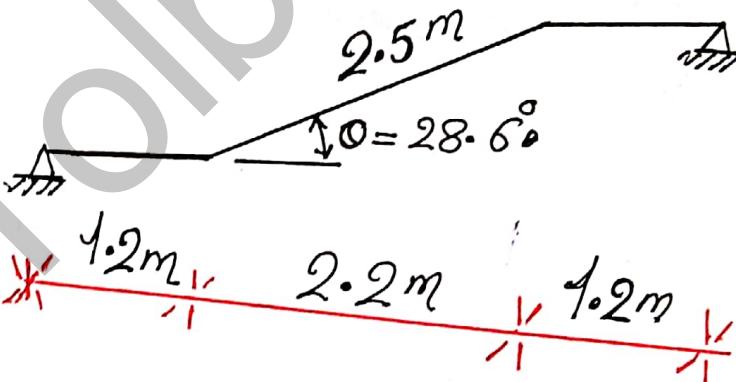
$\approx 200\text{mm}$

$t_s = 200\text{mm}$

$t_{s_{or}} = 270\text{mm}$



 Load Calculation

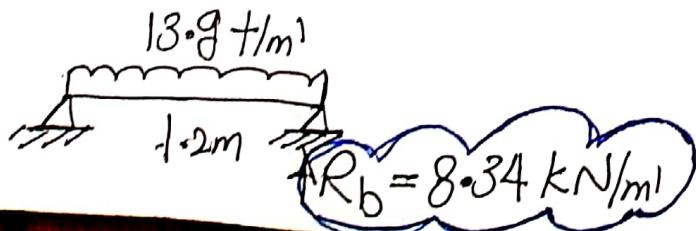


$$w_{sy_{hz}} = 1.4 [0.2 \times 25 + 1.5] + 1.6 \times 3 = 13.9 \text{ kN/m}^2$$

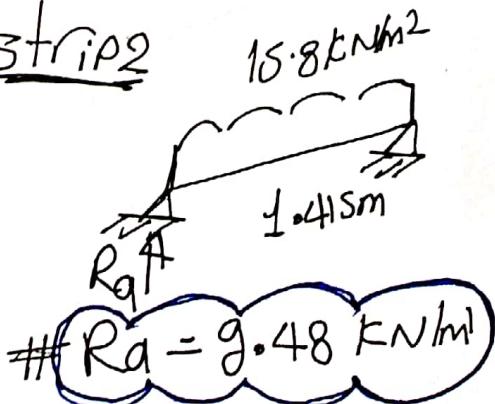
$$w_{sy_{in}} = 1.4 [0.27 \times 25 + 1.5] + 1.6 \times 3 = 15.8 \text{ kN/m}^2$$

 Strip 1

# strip 1



# strip 2

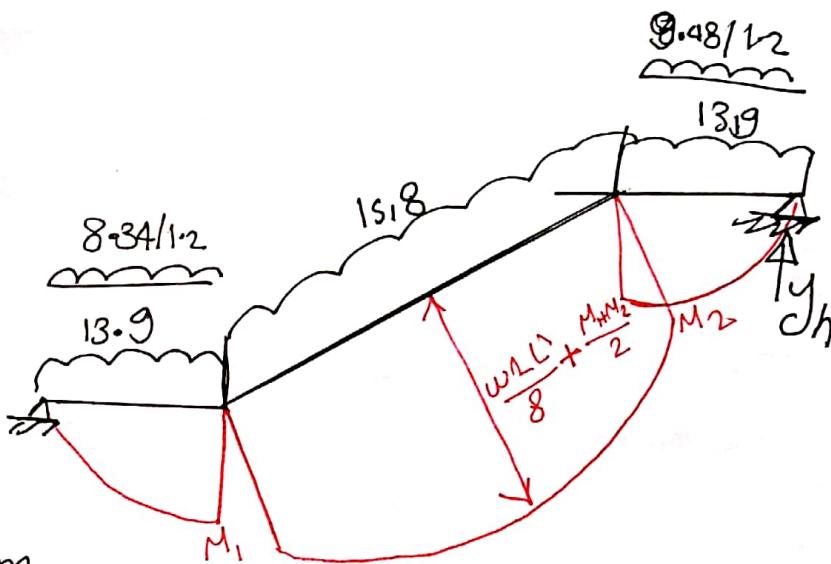


### Strip 3

$$\# y_h = 441.5 \text{ kN/m}$$

$$y_c = 43.1 \text{ kN/m}$$

$$M_{max} = 48.86 \text{ kN.m}$$



$$\rightarrow d = C_1 \sqrt{\frac{M_{ul}}{f_{u,b}}} \rightarrow C_1 = 4.07 \rightarrow J = 0.808$$

$$\# A_s = \frac{M_{ult}}{F_y \cdot J \cdot d} = 933,1836 \text{ mm}^2 \approx 10 \# 12/\text{m}^1 \\ \approx 5 \# 16/\text{m}^1$$

\* ----- \* ----- \* ----- \*

# For Beam B<sub>1</sub> (300x600)

# No torsion

# Var. loads g-

$$w_1 = o \cdot w + w_{all} + R$$

$$= 1.4 \times 0.3 \times 0.6 \times 25 + 2 + 43.1 = 55.7 \text{ kN/m}$$

$$w_2 = o \cdot w + w_{all} -$$

$$= 12.6 \text{ kN/m}$$

#  $M_{max} = 51.45$

$$d = c_1 \sqrt{\frac{M_u \cdot b^6}{f_{cu} \cdot b}}$$

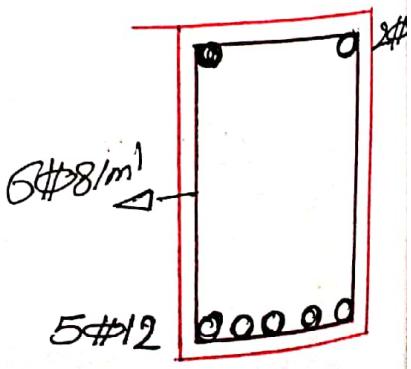
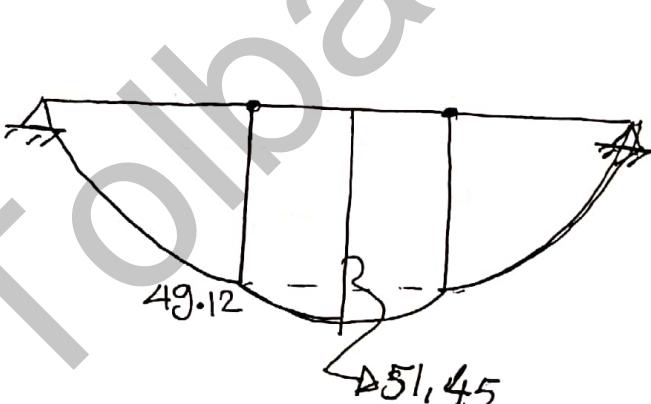
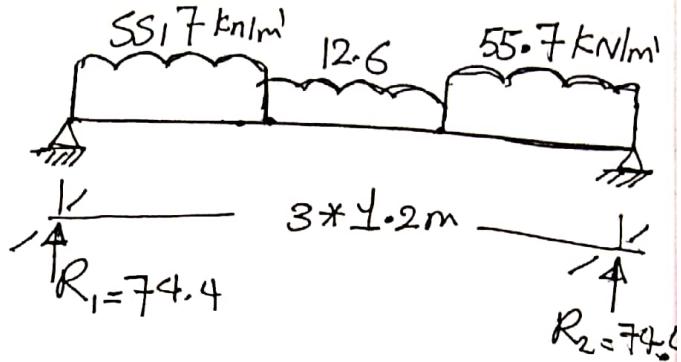
$$\rightarrow 550 = C \sqrt{\frac{51.45 \times 10^6}{25 \times 300}} \rightarrow J = 0.826$$

$$\rightarrow A_s = A_{smin} = \frac{1.1}{F_y} b d = 505 \text{ kN/mm}^2 = \boxed{5\#12}$$

# check shear g-

$$q_r = \frac{Q}{B \cdot J} = \frac{74.4 \times 10^3}{300 \times 600} = 0.45 \text{ N/mm}^2 < q_{rue}$$

# use min. stirrups 5#8/m<sup>1</sup>



$\left[ \begin{array}{c} 18 \\ 2 \end{array} \right] (0.3 \times 600) \rightarrow \text{No torsion}$

$$\#N_1 = 0 \cdot N + w_{\text{wall}} + R \\ = 55.7 \text{ kN/m}^1$$

$$\#N_2 = 12.6 \text{ kN/m}^1$$

$$R = \frac{\sum N * L}{2} = 75.8 \text{ kN}$$

$$\#M_{\text{max}} = 53.53 \text{ kN.m}$$

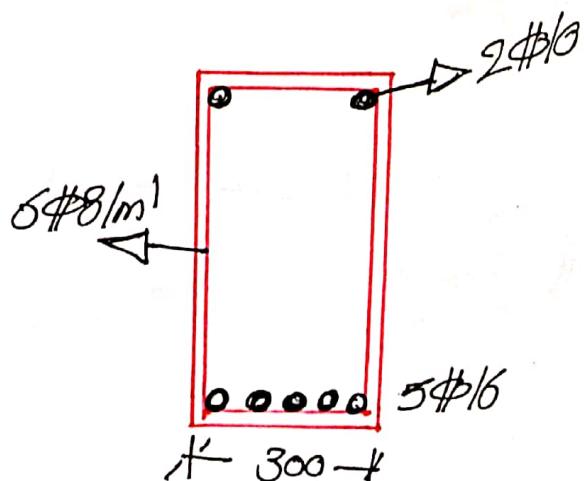
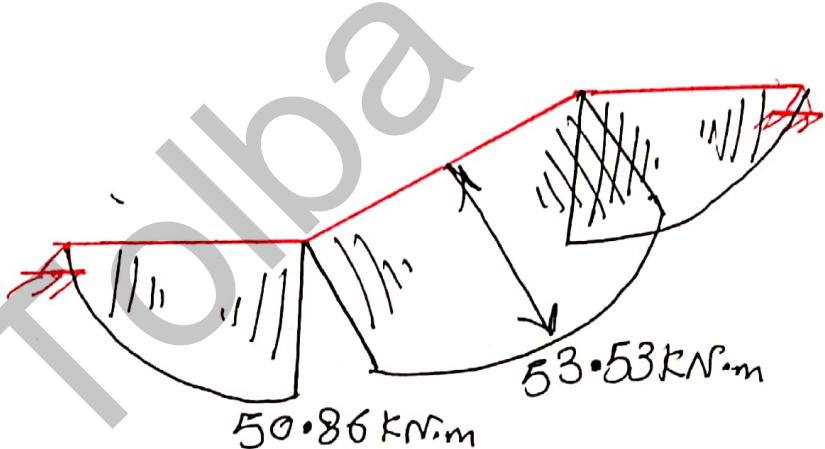
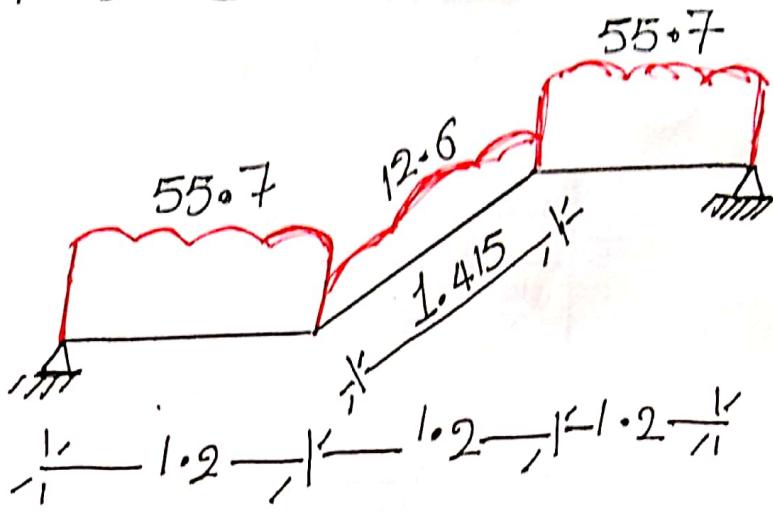
$$d = C_1 \sqrt{\frac{M_u}{f_{\text{cu}} \cdot b}}$$

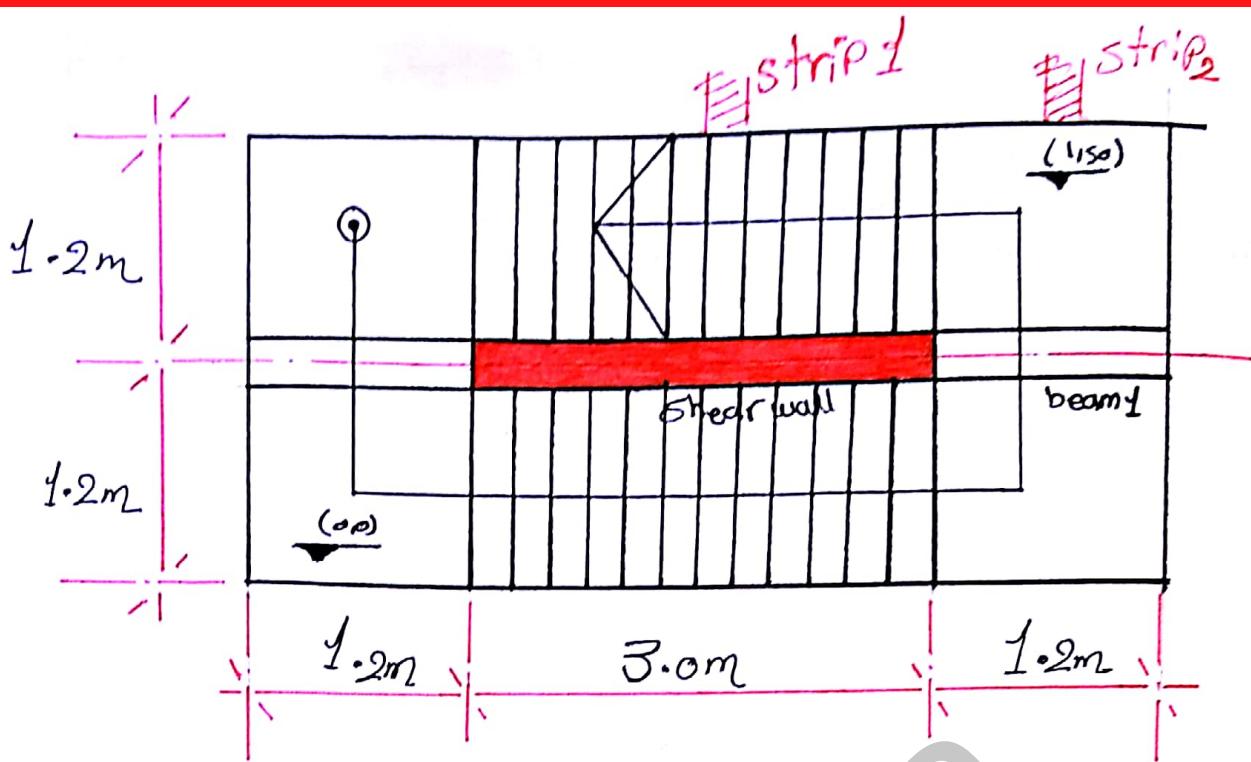
$$J = 0.826$$

$$\#A_s = A_{s\min} = 5 \#12$$

$$P_{\text{max}} = 75.8 \text{ kN}$$

$\rightarrow$  use min stirrups  $6 \#8/\text{m}^1$





Concrete Dimensions :-

$$\# t_s = \frac{1200}{70} = 120\text{mm} \approx 150\text{mm}$$

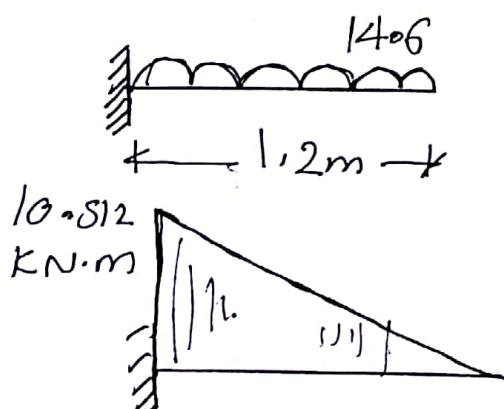
② Load Calculation :-

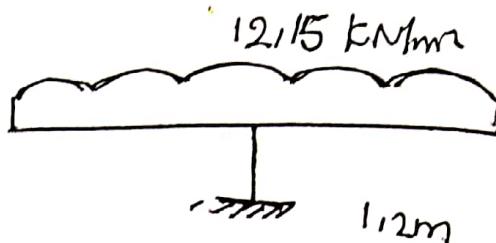
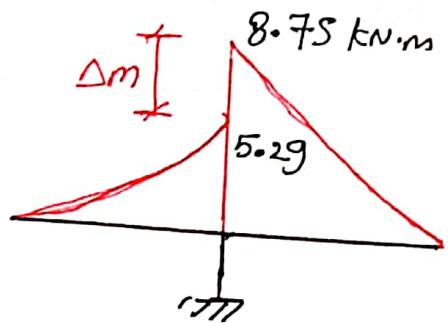
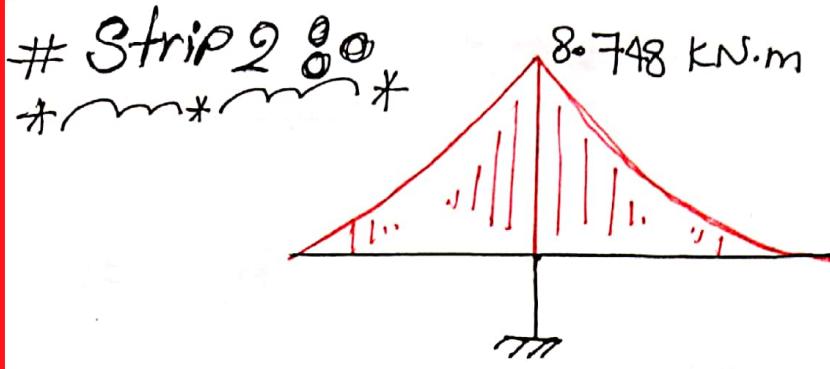
$$n_{S_u}^{h_z} = 1.4[0.15 \times 25 + 1.5] + 1.6 \times 3 = 12.15 \text{ kN/m}^2$$

$$\frac{n_{S_u}}{I_{nc}} = 1.4[0.22 \times 25 + 1.5] + 1.6 \times 3 \times G_S (26.57) = 14.6 \text{ kN/m}^2$$

③ Strips :-

\* \* \* \* \*  
strip 4





$$\Delta m = 3.46 \text{ kN.m}$$

→ Design Slab

$$M_{\max} = 10.512 \text{ kN.m}$$

$$d = C_1 \sqrt{\frac{M_{\text{ult}}}{f_{\text{cu},b}}} \rightarrow 130 = C_1 \sqrt{\frac{10,512 \times 10^6}{25 \times 1000}}$$

$$A_s = \frac{M_{\text{ult}}}{F_y \cdot J \cdot d} = 271 \text{ mm}^2$$

$$C_1 = 0.4 \rightarrow J = 0.826$$

$$A_{s\min} = \frac{l_1}{F_y} b \cdot d = 400 \text{ mm}^2 \simeq 5 \text{ #12 / m}$$

## # Design of Beam $(30 \times 60) \text{ cm}^2$

#  $M = \sigma \cdot w + \text{wall} + R = 1.4 \times 0.3 \times 0.6 \times 25 + 2 + 29 \cdot 16 = 37.46 \text{ kN}$

